

## Appendix B Stoplog Design Examples

The opening for the closure in this example is 47 ft, 4 in. wide by 4 ft, 4 in. high. The closure will be designed using aluminum stoplogs (Example B.1.a.) and steel stoplogs (Example B.1.b.).

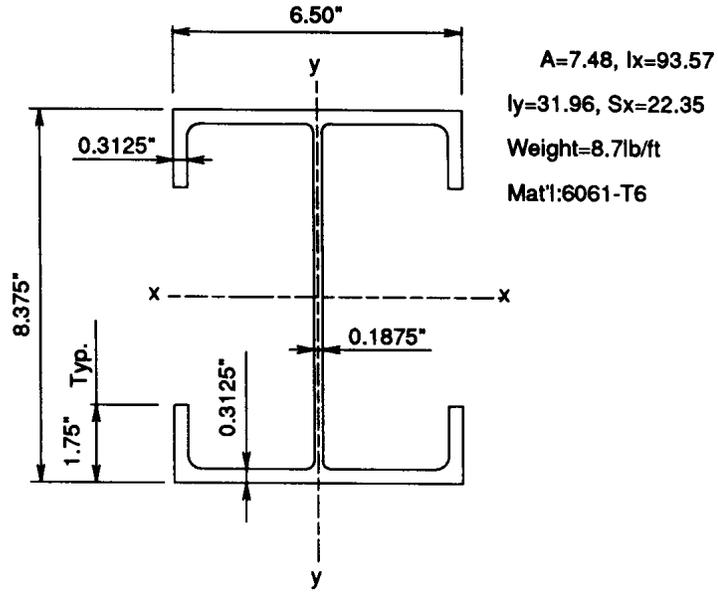
### Load Cases:

The load cases given in EM 1110-2-2502 for inland floodwalls were taken into consideration. The only significant case for design was found to be case I2.

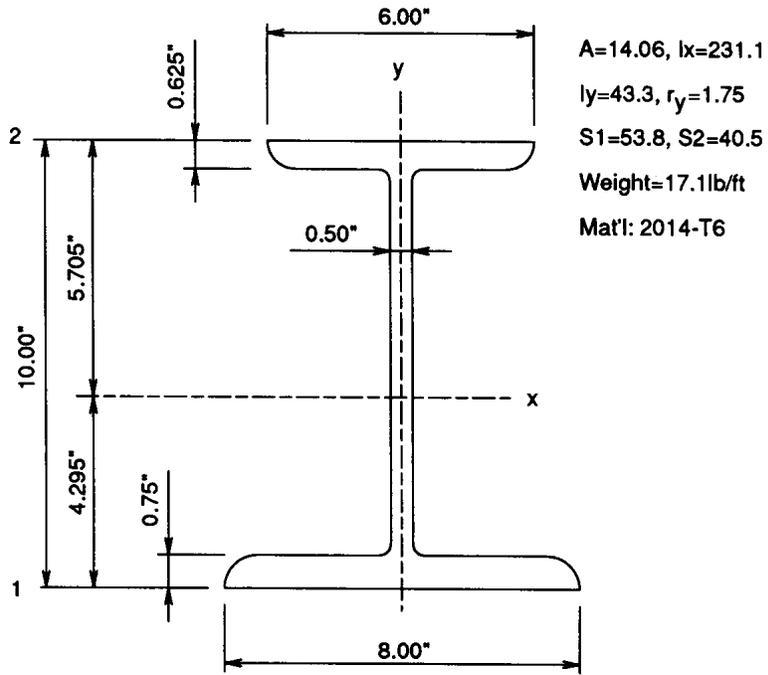
Case I2, Maximum Flood Loading. Stoplogs are in place, water on the unprotected side is at top of closure, water on protected side is at or below the top of sill. Design stresses for aluminum shall not exceed 1.11 times the allowable stresses given in Aluminum Association, Inc. (1986). Design stresses for steel shall not exceed 1.11 times the allowable stresses given in AISC (1989).

The stoplogs are designed as simple beams spanning the openings between posts and the openings between walls and posts. Intermediate posts are designed as cantilever beams fixed at the sill.

EXAMPLE B.1.a.(ALUMINUM).

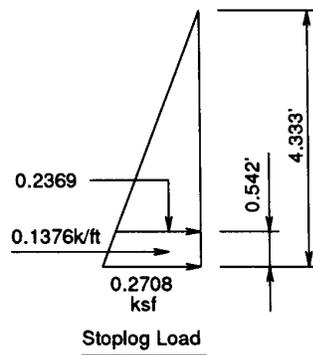


STOPLOG SECTION



POST SECTION

EXAMPLE B.1.a.(ALUMINUM)



STOPLOG:

$$W=0.1376\text{k/ft}, L=\text{Post Spacing}=12.00'$$

$$M=WL^2/8=2.477\text{k-ft}=29.724\text{k-in}$$

$$V=WL/2=0.1376 \times 12/2=0.826\text{k}$$

All Specifications Listed Below Are From  
Table 3.3.27 Of Aluminum Association, Inc. (1986).

Specification 1: Flexural Tension Allowable=19.00ksi

(Nonwelded Member)

Note: The Allowable Stress For Welded Aluminum

Is About 50% Less Than The Nonwelded Member

Specification 15: Compression In Flange Lips.

$$b/t=(1.75-0.3125)/0.3125=4.6 < 6.8, F_{bc}=21\text{ksi}$$

Specification 16: Compression In Flange, Both Edges Supported.

$$b_L=1.75-0.3125=1.4375", b_f=(6.5-2 \times 0.3125-0.1875)/2=2.844"$$

$$b_f/3=0.948" < b_L \text{ o.k.}$$

$$b_f/t=2.844/0.3125=9.1 < 21, F_{bc}=21\text{ksi}$$

Specification 18: Compression In Web.

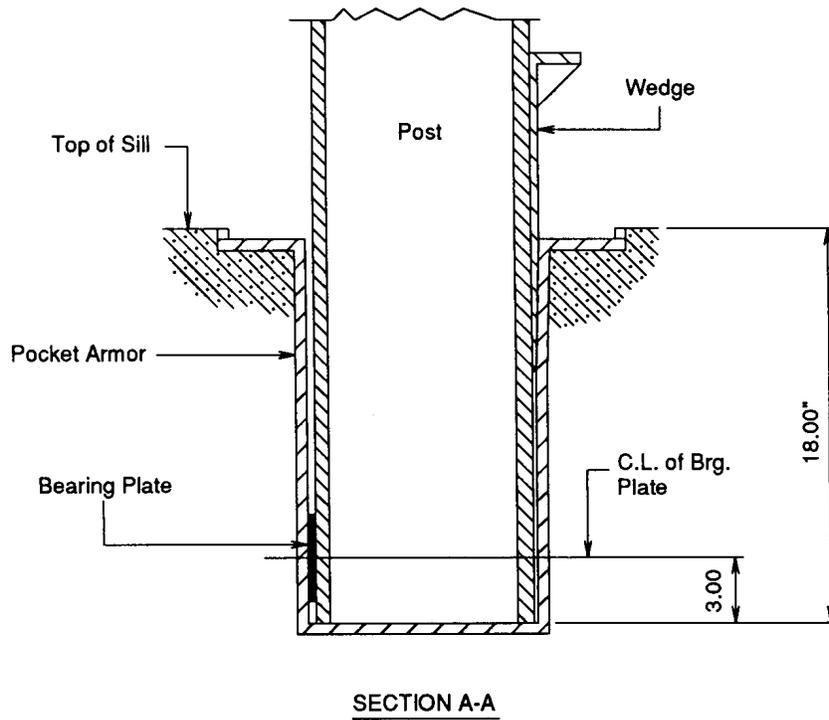
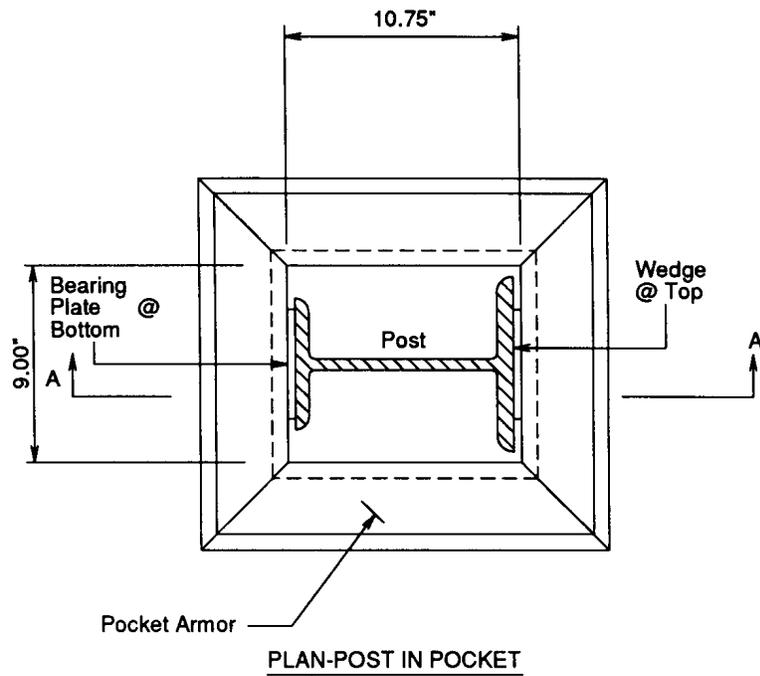
$$h/t=(8.375-2 \times 0.3125)/0.1875=41.33 < 46, F_{bc}=28\text{ksi}$$

$$f_b=M/S_x=29.724/22.35=1.33\text{ksi}, F_b=1.11 \times 19=21.09\text{ksi} > f_b \text{ o.k.}$$

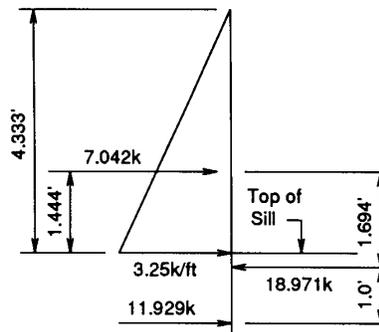
$$f_s=0.826/(8.375 \times 0.18375)=0.526\text{ksi} \text{ Shear Is Negligible.}$$

THIS STOPLOG SECTION PROVIDES AN OVERDESIGN. HOWEVER, IT IS AVAILABLE FROM THE USING AGENCY'S INVENTORY AND CAN BE EASILY INSTALLED BY MANUAL LABOR. IT WILL BE USED.

EXAMPLE B.1.a.(ALUMINUM)



EXAMPLE B.1.a.(ALUMINUM)



INTERMEDIATE POST:

$$M=7.042 \times 1.694=11.93 \text{ k-ft}=143.16 \text{ k-in}$$

$$V=11.929 \text{ k}$$

All Specifications Listed Below Are From  
Table 3.3.8 of Aluminum Association, Inc. (1986).

POST LOADING

Specification 1: Flexural Tension Allowable=25ksi

Specification 11: Flexural Compression.

$$L_b=4.333+1.25=5.583'=67", L_b/r_y=67/1.75=38>19$$

$$F_{bc}=36.3-0.23 \times 38=27.56 \text{ ksi}$$

Specification 18: Compression in Web.

$$h=10", t_w=0.50", h/t_w=20<43, F_{bw}=41 \text{ ksi}$$

Specification 20: Web Shear.

$$h/t_w=20<32, F_s=19 \text{ ksi}$$

$$f_b=M/S=143.16/40.5=3.53 \text{ ksi}, F_b=1.11 \times 25=27.75 \text{ ksi} > f_b \text{ o.k.}$$

$$f_s=11.929/(10 \times 0.5)=2.386 \text{ ksi}, F_s=1.11 \times 19=21.09 \text{ ksi} > f_s \text{ o.k.}$$

THIS POST SECTION PROVIDES AN OVERDESIGN. HOWEVER, IT IS AVAILABLE FROM THE USING AGENCY'S INVENTORY AND CAN BE EASILY INSTALLED BY HAND. IT WILL BE USED.

EXAMPLE B.1.a. (ALUMINUM)

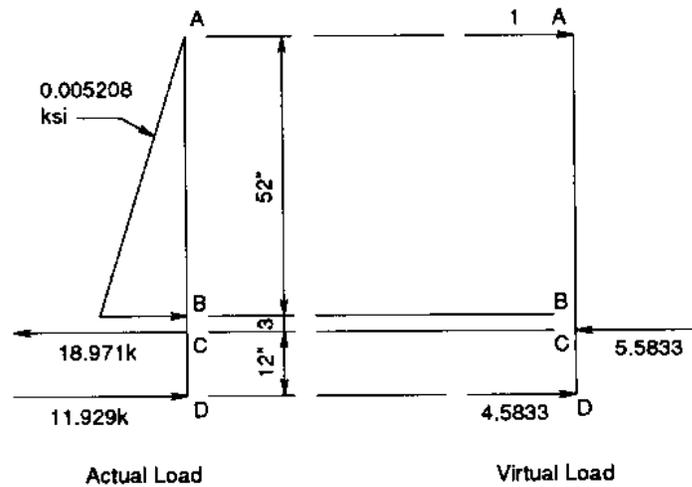
STOPLOG DEFLECTION:

$$\Delta = \frac{5WL^4}{384EI}, W=0.1376/12=0.011467\text{k/in}, E=10800\text{ksi}$$

$$I=93.57, \Delta = 5 \times 0.011467(12 \times 12)^4 / (384 \times 10800 \times 93.57) = 0.064"$$

$$\Delta / L = 1/2250 < 1/200 \text{ o.k.}$$

POST DEFLECTION:



M=Moment Due To Actual Load    m=Moment Due To Virtual Load

$$\Delta A = \sum \int M m d y / E I$$

From A To B:  $M=0.000868y^3, m=y, Mm=0.000868y^4$

$$\int_0^{52} M m d y = 66003$$

From D To C:  $M=11.929y, m=4.5833y, Mm=54.6742y^2$

$$\int_0^{12} M m d y = 31493$$

From C To B:  $M=143.148-7.042y, m=55-y$

$$Mm=7873.14-530.458y+7.042y^2$$

$$\int_0^3 M m d y = 21296$$

$$\Delta A = (66003+31493+21296)/(10800 \times 231.1) = 0.048" \quad \Delta / L = 1/1396 \text{ o.k.}$$

EXAMPLE B.1.b. (A36 STEEL)

All Equation Numbers In Parentheses Are  
From The Specification In AISC (1989)

STOPLOG:

$$\text{Trial Section S4x7.7, } I_x=6.08, S_x=3.04, b_f = 2.663" = 0.2219'$$

$$W = \text{Load On Bottom Log} = 0.5[2 \times 0.2708 - 0.0625 \times 0.2219](0.2219)$$

$$W = 0.0586 \text{ k/ft}, M = 0.0586(12)^2/8 = 1.055 \text{ k-ft} = 12.66 \text{ k-in}$$

$$f_b = M/S_x = 12.66/3.04 = 4.164 \text{ ksi}$$

$$F_b = 0.66F_y = 24 \text{ ksi (F1-1)}$$

$$\triangle = 5(0.0586/12)(12 \times 12)^4 / (384 \times 29000 \times 6.08) = 0.155"$$

$$\triangle / L = 1/929 < 1/200 \text{ o.k.}$$

INTERMEDIATE POST:

Post Pocket Details Will Be Similar To Those

Used In Example B.1.a.

$$\text{Trial Section W8x10, } I_x=30.8, S_x=7.81, b_f = 4", r_T = 0.99$$

$$L_b = 67", L_c = 76 \times 4/6 = 50.67 < 67 \text{ (F1-2)}$$

$$L_b/r_T = 67/0.99 = 68$$

$$F_b = [2/3 - 36(68)^2 / (1530 \times 10^3)] \times 36 = 20.08 \text{ ksi (F1-6)}$$

$$M = 143.16 \text{ k-in (Same As Example B.1.a.)}$$

$$f_b = M/S_x = 143.16/7.81 = 18.33 \text{ ksi}$$

$$1.11F_b = 22.31 \text{ ksi} > f_b \text{ o.k.}$$

$$\triangle \text{ (From Example B.1.a.)} = 0.048"$$

$$\triangle \text{ (For Steel Post)} = 0.048(10800/29000)(231.1/30.8) = 0.134"$$

$$\triangle / L = 0.134/67 = 1/500 < 1/200 \text{ o.k.}$$

USE S4x7.7 STOPLOGS AND W8x10 POSTS